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$2 \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots$, where $2 \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots$

$(2 \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots)$ Substituting (3.4.4) into

(3.4.2) we have, $2 \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots$ We

can find the optimum solution by $0 = \dots$

$a \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots$, by some operations leads to $0 \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots$

$-\left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots$ $a \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots$, therefore we

have $2 \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) \left(\frac{2}{L} \right) = \dots$ for $0 > b$ Substitute it back

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to (3.4.4), we have a $C_b = 2$ (3.4.5) That means the optimum cross-section for elliptical ...

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have $2 C a = \text{for } 0, > b a$ Substitute it back to (3.4.4), we have a $C b == 2 2$ (3.4.5) That means the optimum cross-section for elliptical shapes is a circle.

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(2) $L C = (3.4.4)$ Substituting (3.4.4) into (3.4.2) we have, $22 aCaAellp - =$ We can find the optimum solution by $0 =$ $a A$, by some operations leads to $0^2 22 22 =$ $- - = aC aC a A$, therefore

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7.2 A bar is built-in at the left end and supported at the right end by a linear spring with spring constant k . Find the equation for buckling loads. Hint: The boundary conditions are $w = 0$ and $\theta = 0$ at the left end; and $w = 0$ and $M = kV$ at the right end.

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(3) Two approaches are employed to find the solution. (i) Assume that the bending stress reaches the allowable allowable first and find the corresponding bending maximum bending moment.

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up in three ways: (1) to solve for the displacements first and then the stresses; (2) to solve for the stresses first and then the displacements; and (3) to solve for both stresses and displacements simultaneously. These three methods are identified in the literature as (1) the displacement method, (2) the stress or force method, and (3) the

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combined or mixed method. Closed form solutions of the partial differential equations with their complicated boundary conditions for any of these three methods have been obtained only in special cases. In order to obtain solutions, various special methods have been developed to determine the stresses and displacements in structures. The

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equations have been reduced to two and one dimensional forms for plates, beams, and trusses. By neglecting the local effects at the edges and ends, satisfactory solutions can be obtained for many case~. The procedures for reducing the three dimensional equations to two and one dimensional equations are described in Chapter 1,

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